

A Numerical Comparison between Spiral Transverse RC and CFST Columns under Loads of Varying Eccentricities

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Abstract

This paper is the first to present the results of a numerical comparison between the performances of two newly developed concrete columns namely: multi-tied spiral transverse reinforced (MTSTR) column and concrete-filled steel tube (CFST) column under eccentric compressive loads. The behavior of MTSTR columns under eccentric loads has not been studied until today and also this behavior is not compared to that of the CFST columns. The numerical models of these columns were constructed using the nonlinear finite element method and validated against the previously published experimental data in the literature. Concrete damage plasticity model and elastic-perfect plastic model were used to simulate the behavior of concrete core and steel of the columns, respectively. This provides the capability of modeling of the nonlinear large deformations of the columns. The obtained results show that the MTSTR columns can provide greater load carrying capacity, ductility, and energy absorption with slightly lower initial stiffness than the CFST columns under the same eccentricities. For instance, the load-carrying capacity of MTSTR column is 18 percent greater than that of the CFST column when the load eccentricity is 100 mm. In case of 100 mm eccentricity, the ductility of the improved version of the MTSTR column proposed in this study is 30 percent greater than CFST one.

Keywords

multi-tied spiral transverse reinforced column, concrete-filled steel tube column, numerical analysis, eccentric load, load carrying capacity

1 Introduction

Columns have different performances in buildings depending on the loading conditions. Under vertical loads, columns behave as axial members and the axial stiffness and strength play a key role in the overall behavior of the building [1, 2]. In this situation, ductility and energy dissipation capability of the columns are not considered as essential criteria in the design codes. However, under the lateral loads, column behaves as an axial-flexural-shear member and the flexural and shear stiffness play main roles in addition to its axial stiffness in the building performance [3–10]. Under this circumstance, ductility, and energy dissipation of the column cannot be ignored in the analysis and design procedures. This issue becomes more highlighted when the height of the building increases. In tall buildings, columns are subjected to significant axial, shear and bending moments due to the combination of gravity and wind or seismic loads [11–13].

The bearing capacity and deformability of the column are affected by configurations of the column cross-section [14–21]. The lack of load capacity, inadequate ductility, and low energy absorption are known as the most effective parameters on the column failure [22, 23]. Accordingly, several researchers attempted to improve the load capacity, ductility, and energy absorption of the column under different loads by means of confinement of the concrete core with different methods. An overview of recent literature shows that utilization of concrete-filled steel tubes (CFST) and transverse reinforcement (TR) are two effective ways to improve the confinement of concrete core and increase of the load carrying capacity, ductility, and energy absorption.

The results of the previous experimental studies show that the ductility and strength of CFST columns are more than those of traditional columns [24–30]. Lam and Williams [31] examined the performance of CFST columns

experimentally subjected to axial compressive loads and found that the increase of the compressive strength of concrete leads to an increase in the bearing capacity of the column. Sakino et al. [32] presented stress-strain relationships for concrete of the CFST columns under the axial loads through an experimental investigation. The eccentric compressive loads are also induced in the columns during the earthquake [33]. In an experimental research, Xiong et al. [34] evaluated the bearing capacity of the CFST columns under eccentric loads. They found that as the yield stress of the steel tube increases, the bearing capacity of the CFST columns increases. In Lee et al.'s investigation [35], eccentric load was applied to the short CFST columns. In addition to experimental works, many researchers have studied the behavior of CFST columns using analytical and numerical methods. Wang et al. [36] simulated the CFST columns wrapped with the CFRP fibers under eccentric load and concluded that the CFST columns with CFRP fibers have greater bearing capacity and ductility compared to those of regular CFST column. In a research carried out by Li et al. [37], eccentric loads were applied to the CFST short columns in laboratory. They also simulated the behavior of these columns using finite element method. They found that the most significant effect of the confinement concrete is observed in the corner of the pillow. They also derived the change of the force-moment interaction diagrams of this type of columns against different parameters and presented some relationships for calculation of the maximum bearing capacity of CFST column. Liu et al. [38] studied the CFST columns with different cross-sections under axial load using a laboratory and a finite element method. Their investigation presented that CFST columns with a T-shaped cross-section are more efficient than CFST columns with an L-section.

Tran and Li [39–42] used different configurations of transverse reinforcement in concrete columns and examined the performance of the constructed columns under cyclic loads. They proposed a formula to predict the initial stiffness of the RC columns. The spiral reinforcement was used for the first in the bridge columns. The load capacity of bridge's column were increased by using spiral reinforcement. In recent years, the use of spiral reinforcement in the columns of the residential buildings has been of great interest to the researchers. Jing et al. [43] applied compressive load to the column with spiral reinforcement in the laboratory. They proposed a new configuration of spiral reinforcement columns called MTSTR (multi-tied-spiral transverse reinforcement). In another experimental

research carried out by Li et al. [44], the bearing capacity of the column with MTSTR under axial compression has been investigated. They proposed a stress-strain model for confined concrete used in the MTSTR column [45].

In the above-mentioned previous studies, the ductility and energy absorption of the MTSTR columns were investigated only under the axial loads. Moreover, these characteristics of the MTSTR and CFST columns have not been compared with each other until today. This research is an attempt to overcome to these shortcomings.

2 Numerical simulation

MTSTR and CFST columns were modeled numerically using the ABAQUS/Standard software. Several steps were needed for develop each column FE model. These steps include the geometry description, definition of the material behavior, determination of the element types and sizes, definition of the loading scenarios, determination of the boundary conditions and interactions, and finally the analysis type. These steps are briefly described for each column type in Subsections 2.1 and 2.2.

2.1 Simulation of MTSTR column

2.1.1 Geometry description

Rectangular concrete short column, spiral transverse reinforcement, longitudinal reinforcement, rigid cap and rectangular stirrup are the essential ingredients of the MTSTR column. The rectangular hoops were generated as wire (truss elements). In addition, rectangular concrete column, rigid cap, longitudinal reinforcement, and spiral reinforcement were simulated as solid. All components were assembled to develop the MTSTR short column (See Fig. 1(a)). The longitudinal, spiral transverse and rectangular stirrup reinforcements were placed into the concrete short column. Fig. 1(b) displays a view of the arrangement of all the reinforcements in the column. A rigid cap is placed on the top of the column to apply a desired eccentric axial compression load on the column. Moreover, another rigid cap was located on the bottom of the column to apply the boundary condition of the column. As illustrated in Fig. 1(b), the concrete was confined by the spiral reinforcement.

2.1.2 Material properties

Two types of material, namely concrete and steel are used to simulate MTSTR column. The longitudinal, spiral transverse and rectangular stirrup reinforcements and rigid caps were considered as steel and short column was considered as concrete. Concrete damage plasticity model

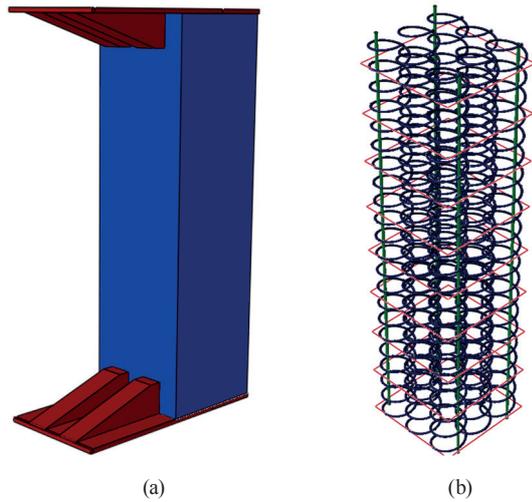


Fig. 1 Details of MTSTR column a) a view of MTSTR column b) arrangement of reinforcement

was used to define the concrete behavior. This material model has been used to take into account the concrete plasticity and damages. Several input parameters such as uniaxial compressive and uniaxial tensile stress-strain relationships and plasticity parameters have to be defined for the CDP model. Uniaxial compressive stress-strain relationship of concrete is defined using in Eqs. (1)–(7) [46–48].

$$\frac{\sigma_c}{\sigma_o} = \begin{cases} 2x - x^2 & x \leq 1 \\ \frac{x}{\beta_0(x-1)^{\eta_0} + x} & x > 1 \end{cases} \quad (1)$$

$$x = \frac{\varepsilon_c}{\varepsilon_0} \quad (2)$$

$$\sigma_o = f_c \quad (3)$$

$$\varepsilon_0 = [(1300 + 12.5 \times f_c) + 800 \times \theta^{0.2}] \times 10^6 \quad (4)$$

$$\eta_0 = 1.6 + \frac{1.5}{x} \quad (5)$$

$$\beta_0 = f_c^{0.1} / (1.2\sqrt{1+\theta}) \quad (6)$$

$$\theta = A_s f_y / A_c f_c \quad (7)$$

In Eq. (1), two coefficients β_0 and η_0 are used to calculate stress-strain relations of concrete in compression state. β_0 and η_0 are calculated using Eqs. (5) and (6). Also, in formula (Eq. (1)), σ_c is the compressive stress of concrete and σ_o denotes the maximum compressive stress of concrete. In Eq. (2), ε_0 is the maximum compressive strain of concrete and ε_c is the compressive strain of concrete. In the Eq. (7), A_s defines the cross-sectional area of steel,

f_y is the yield stress of steel and A_c stands for the concrete cross-sectional area. Uniaxial tensile behavior of concrete was defined using in Eqs. (8)–(11) [46–48].

$$\frac{\sigma_t}{\sigma_p} = \begin{cases} 1.2x - 0.2x^6 & x \leq 1 \\ \frac{x}{0.31\sigma_0^2(x-1)^{1.7} + x} & x > 1 \end{cases} \quad (8)$$

$$x = \frac{\varepsilon_t}{\varepsilon_p} \quad (9)$$

$$\sigma_p = 0.26(1.2 \times f_c)^{2/3} \quad (10)$$

$$\varepsilon_p = 43.1\sigma_p \times 10^6 \quad (11)$$

In Eq. (9), σ_p is the maximum tensile stress of concrete and σ_t is the tensile stress of concrete. In Eq. (9), ε_p is the maximum tensile strain of concrete and ε_t denotes the concrete tensile strain [46–48]. The optimum values of the plasticity input parameters of the CDP model including the dilation angle (ψ), eccentricity (ϵ), the ratio of the biaxial compressive strength to the uniaxial compressive strength of the concrete (f_{b0}/f_{c0}), the ratio of the tensile meridian to the compressive meridian in the deviatoric plane of the yield surface (K), and viscosity (μ) were introduced in Table 1 [49–61].

An elastic-plastic material model was considered to simulate the behavior of steel [62, 63]. Some parameters such as modulus of elasticity, Poisson's ratio, yield, and ultimate stress of steel are utilized to define the elastic-plastic behavior of steel. This material model was assigned to all reinforcements and rigid caps. The modulus of elasticity of rigid caps was considered eight times larger than that of reinforcement, to increase the rigidity of the rigid caps. The material characteristics of concrete and steels used in this study is outlined in Table 2.

2.1.3 Type and size of elements

Three dimensional eight-node brick elements (C3D8R) were used to simulate the concrete, spirals, longitudinal bars, and rigid caps. The rectangular hoops were modeled using two-node truss elements, T3D2. According to the results obtained from several sensitivity analyses, the size of the elements of concrete and steel parts were chosen as 50 mm and 25 mm respectively.

Table 1 Plasticity Parameters of Concrete

Dilation Angle	Eccentricity	f_{b0}/f_{c0}	K	Viscosity Parameter
40°	0.1	1.16	0.66	0.0001

Table 2 Material properties of concrete and steels

Material	Yield Stress (MPa)	Ultimate Stress (MPa)	Elastic Modules (MPa)
Concrete	-	41	3×10^4
Steel of spirals	1196.7	1349.7	2.1×10^5
Steel of longitudinal bars	344.7	503.6	2.1×10^5
Steel of stirrups	344.7	503.6	2.1×10^5

2.1.4 Loading, boundary conditions and interactions

Rectangular stirrup reinforcement, longitudinal reinforcement and spiral reinforcement were embedded in the concrete. In other words, the displacements of each node of all reinforcement were regarded the same as those of the concrete node. The rigid caps were tied to the top and bottom surfaces of the column. Hence, the displacements of the rigid cap at the top of the column were considered the same as those of the top surface of the column. Furthermore, the displacements of the rigid cap at the bottom of the column were considered the same as those of the bottom surface of the column. The compressive loads were applied as the controlled displacements across the two opposite lines on the top and bottom rigid caps to simulate the eccentricity (green lines in Fig. 2). The loading and boundary conditions of the MTSTR column are shown in Fig. 2. As shown in Fig. 2, the displacement of the bottom rigid cap is restrained in all transitional directions. The displacement of the top rigid cap in 1 and 3 directions is restricted ($U1 = U3 = 0$) (see Fig. 2). Rotations of the bottom and top rigid caps about 1 and 2 axes were restrained ($UR3 = UR2 = 0$).

Displacement load($U1=U3=0, UR2=UR3=0$)

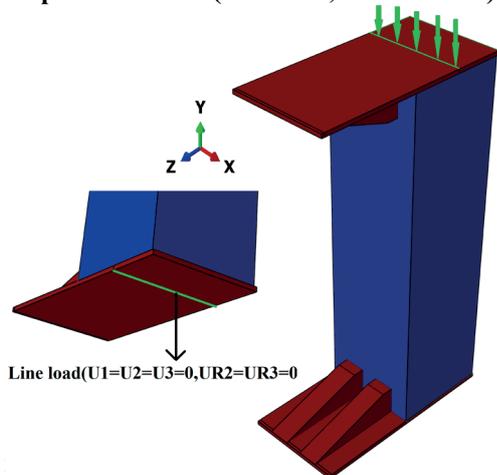


Fig. 2 Loading and boundary conditions of MTSTR column

2.1.5 Analysis

Static analysis was used to obtain the MTSTR column response under axial compressive load with eccentricity. The time period and the maximum number of increment for analysis were considered as 1 and 10000, respectively.

The time period and the maximum number of increment for analysis were considered as 1 and 10000, respectively. The load-displacement diagram of the column was obtained based on the calculated reaction force of bottom rigid cap and applied displacement of the top rigid cap.

2.2 Simulation of CFST column

The CFST column includes a steel tube, rectangular concrete column and, rigid caps. The steel tube was generated as shell elements and other components of the CFST column were generated as solid elements. The concrete column was placed into the steel tube and rigid caps were placed at the top and bottom of the column. Fig. 3 shows a schematic view of the CFST column. The material behavior of the steel tube was considered as the steel behavior described in Subsection 2.1.2. The material behaviors of the other components of the CFST column were considered the same as those of MTSTR column.

The steel tube was simulated using S4R elements. S4R element is a four-noded shell element and in each node, six degrees of freedom (translational and rotational) were defined. Other components of the CFST column have meshed the same as MTSTR column. The size of the element was considered 50 mm. The steel tube was tied to the concrete column. The loading, interaction and boundary conditions of other components of the CFST column were considered the same as the MTSTR column. Moreover, the analysis procedure of the CFST column resembles that of the MTSTR column.

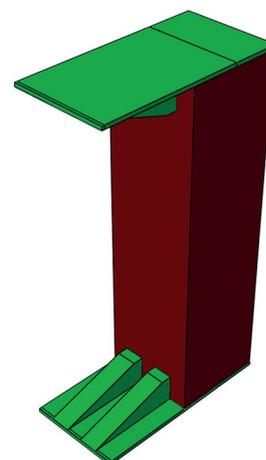


Fig. 3 View of CFST column

3 Verification of numerical models

The results obtained from the models developed in Subsection 2.2 (MTSTR column and CFST column) were compared to those of the literature to investigate the validity of the results obtained from the models. The details are described in Subsections 3.1 and 3.2.

3.1 MTSTR column

The behavior of MTSTR column under axial compressive load was investigated by Li et al. [44, 45]. The columns with square shape cross-section were tested in their research. The width (B) and height (L) of the columns were considered as 250 mm and 900 mm, respectively. The rectangular stirrup reinforcement and transverse spiral reinforcement (with a diameter of 4 mm) were considered in their columns. A longitudinal reinforcement with a diameter of 6 mm was placed at each corner of the columns. The diameter and space of the stirrup reinforcement were considered as 6 mm and 100 mm, respectively. Four specimens of MTSTR column with different properties were tested. The details of the MTSTR column specimens such as cross-section type, number of spiral reinforcement, the vertical spacing of the spirals and diameter of the spirals are presented in Table 3. The arrangement of the reinforcement and column type are displayed in Fig. 4. In the research attempts carried out by Jing et al. [43] and Li et al. [44, 45], the diagrams of the axial compressive load-axial strain of the columns were obtained when the specimens were subjected to the axial compressive load (using a jack).

All specimens listed in Table 3 are simulated in this study. The strength of the concrete and reinforcements were considered based on the data presented in Table 2 [43–45]. Moreover, the plasticity parameters of the concrete were considered according to Table 1. In Fig. 5, the results obtained from the numerical models are compared with those of the Jing et al. [43] and Li et al. [44, 45].

As illustrated in Fig. 5, a plausible agreement is seen between results obtained from the numerical model and experimental ones and. Minor differences between tests and models are due to limitation, simplification, and assumption

Table 3 Details of MTSTR column specimen

Specimen	Type of section	Number of spirals	Vertical Spacing of Spirals, S (mm)	The diameter of the spiral, D (mm)
STR1	C	4	40	70
STR2	B	4	40	100
STR3	A	8	40	70
STR4	A	8	60	70

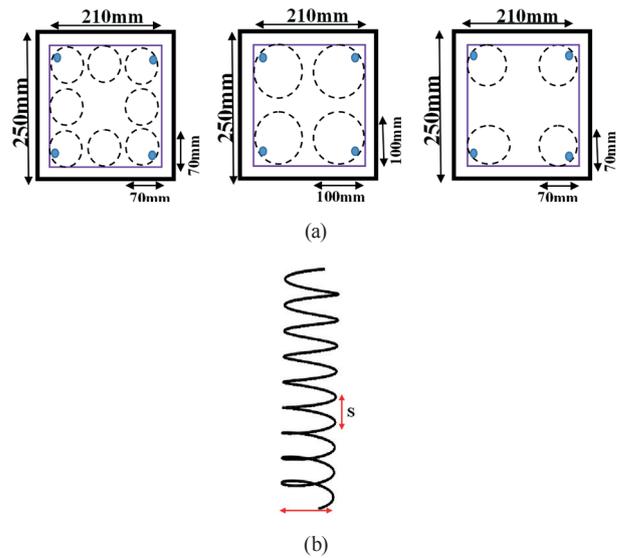


Fig. 4 Details of MTSTR column (a) cross section type (b) spiral properties

Table 4 Ultimate bearing capacity of MTSTR column obtained from model and test

Specimen	Pu(Exp)kN	Pu(FEM) kN	Pu(Exp)/Pu(FEM)
STR1	2900	2960	0.97
STR2	3100	3150	0.98
STR3	3350	3430	0.97
STR4	2900	3165	0.91

considered in the numerical models. Moreover, the ultimate bearing capacity of the column obtained from the numerical model and test are compared in Table 4. As shown in Table 4, the maximum difference between the numerical model (FEM) and test are approximately less than 9 percent.

3.2 Model of CFST column

Li et al. [37] performed an experimental investigation of the behavior of the CFST column under axial load with eccentricity. The CFST column specimens (with square shape cross-section) had width and height of 150 mm and 450 mm, respectively. The details of CFST column specimens are presented in Table 5 [37]. In the research carried out by Li et al. [37], the CFST column specimens were subjected to the eccentric compressive axial load. The numerical models of CFST column specimens presented in Table 5 were simulated in the current study. The concrete plasticity parameters were considered similar to the data presented in Table 3. The diagram of the axial load-axial displacement of the column was obtained from the developed models. A comparison has been made between the results obtained from the models and those of Li et al. [37] (see Fig. 6). As illustrated in Fig. 6, a slight difference is

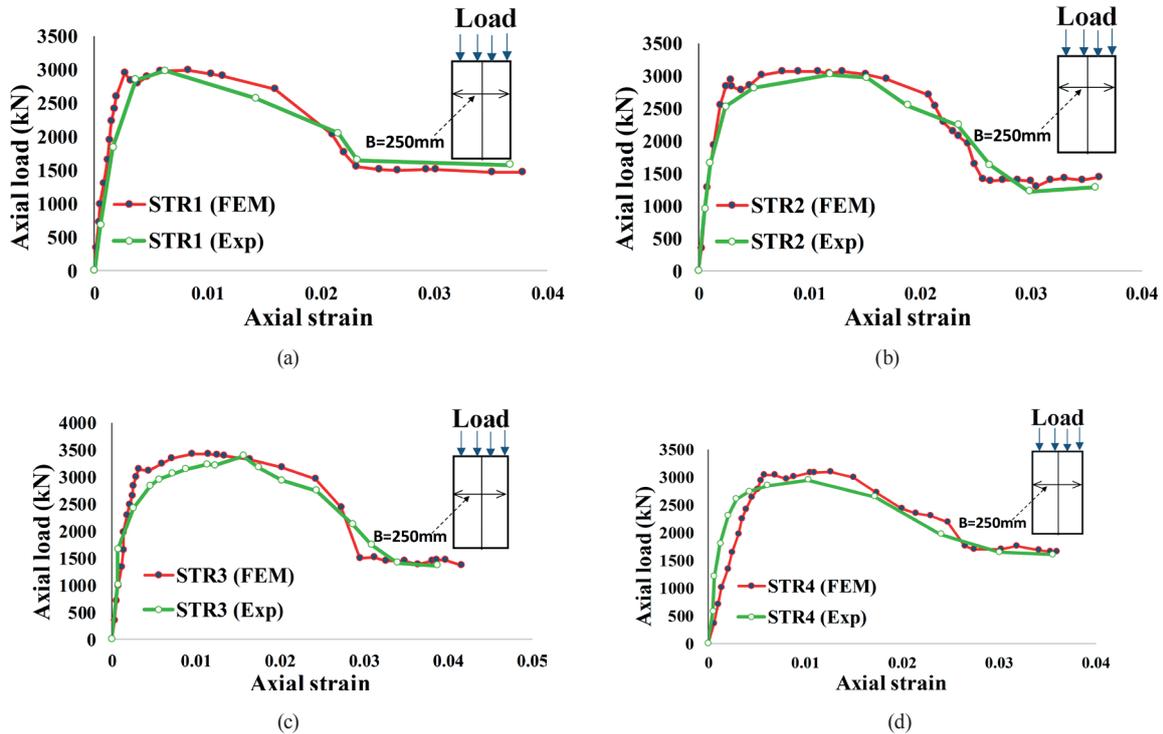


Fig. 5 Comparison between results of MTSTR numerical model and those of literature

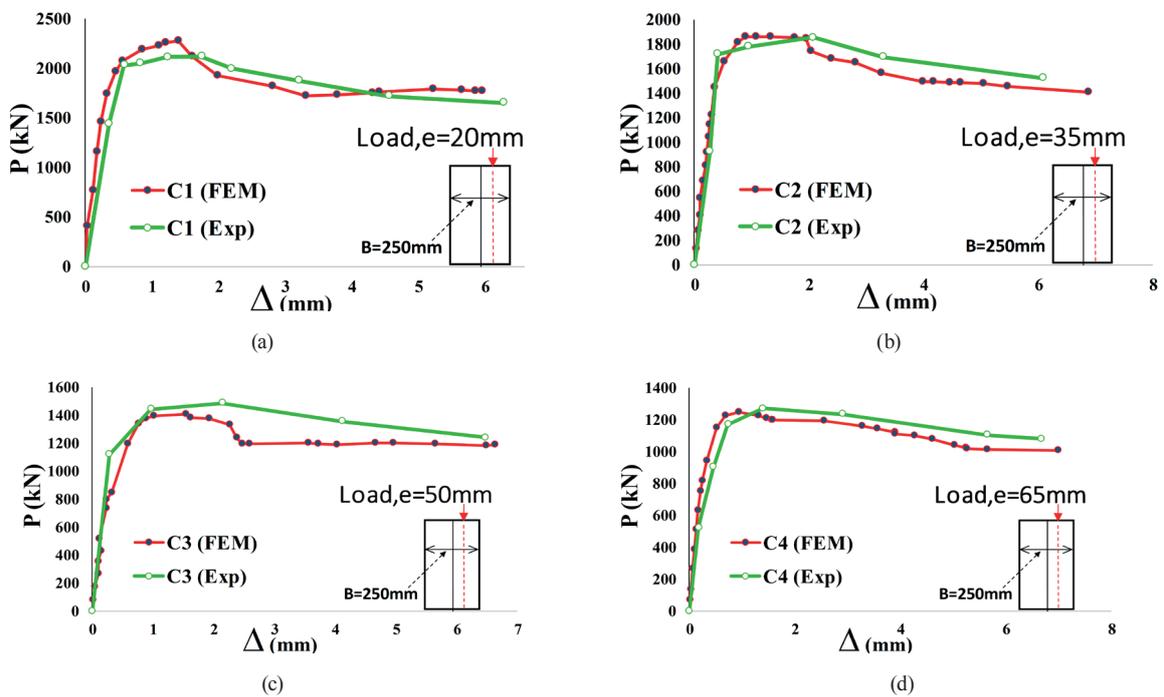


Fig. 6 Comparison between results of CFST numerical model and those of literature

Table 5 Properties of CFST column specimens

Specimen	B (mm)	t (mm)	L (mm)	e (mm)	f_y (MPa)	f_u (MPa)	f_{cu} (MPa)
C1	150	4	450	20	434.56	546.2	88
C2	150	4	450	35	434.56	546.2	88
C3	150	4	450	50	434.56	546.2	88
C4	150	4	450	65	434.56	546.2	88

Table 6 Peak- load of CFST column obtained from model and test

Specimen	Pu(Exp) kN [37]	Pu(FEM) KN	Pu(Exp)/Pu(FEM)
C1	2114.4	2248.5	0.94
C2	1885.9	1864.88	1.011
C3	1561.4	1448.5	1.077
C4	1265	1246.89	1.014

observed between the results obtained from the FEM and the experimental one. It means that the numerical models of the CFST column developed in this study are capable to predict the behavior of these columns under eccentric axial loads. In Table 6, the peak-loads of the CFST column specimens obtained from the models are compared with those of the literature. As shown in Table 6, the maximum difference between the peak loads derived from the numerical models and tests is about 7 percent.

4 Comparison of the performance of the MTSTR and CFST under axial eccentric load

In order to compare the performance of the MTSTR and CFST short columns, three models of MTSTR (STR3, MSTR, and ASTR) and one model of CFST column, were developed and analyzed under axial and eccentric compressive loads. The specifications of the models are summarized in Table 7 and Fig. 7.

Table 7 Details of the specimens simulated in ABAQUS

Specimen	Cross-section	Size of the longitudinal reinforcement	Vertical Spacing of the Spiral, S	Diameter of the spiral, D	size of the spiral	Thickness of the steel tube (mm)	Steel weight (kg)
STR3	F1	4	40	70	4	-	6.2
MSTR	F1	6	40	70	6	-	10.96
ASTR	F1	20	40	70	6	-	18.56
CFST	F2	-	-	-	-	3	21.6

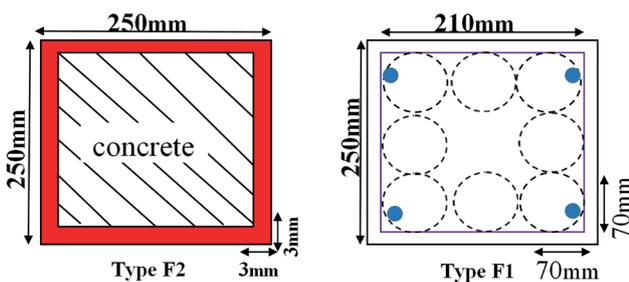


Fig. 7 Cross-sections of the columns constructed in ABAQUS

The bearing capacity of strongest of MTSTR column (STR3) under pure compressive axial load was the same as that of CFST column (about 3430 kN). In the CFST column, the yield and ultimate stresses of the steel were considered as 344 MPa and 503 MPa respectively. In the MTSTR columns, the yield and ultimate stress of the steel were considered the same as outlined in Table 2.

In Table 7, the dimensions of each section are 250 × 250 mm and the height of the all columns is 900 mm. The weight of the steel consumed in each configuration was calculated and indicated in Table 7. The distance between the rectangular hoops is 100 mm and the size of the rectangular hoops is 6 mm. In all MTSTR columns, there are four longitudinal reinforcements, as shown in the cross-sections of the columns (see Fig. 7).

The ductility ($\mu_{0.85}$) was defined based on Eq. (12) and Fig. 8. In Eq. (12), $\Delta_{0.85}$ is the axial displacement of the column at an axial load consistent to 85 % of the maximum axial load on the descending branch of the axial load-displacement curve (see Fig. 8) and Δ_y stands for the axial displacement at the limit of elastic behavior (see Fig. 8) [64]. In Fig. 9, the MTSTR columns having eight spiral reinforcements, and were subjected to the axial and eccentric loads. The force-displacement diagrams of MTSTR columns are compared with the CFST column in Fig. 8.

$$\mu_{0.85} = \frac{\Delta_{0.85}}{\Delta_y} \tag{12}$$

Moreover, the maximum bearing capacity (Pu) and the ductility ($\mu_{0.85}$) of the columns under axial and eccentric loads are presented in Table 8. According to Fig. 9 and Table 8, although the bearing capacity of the STR3 column is close to that of CFST column, the ductility of STR3 column is two times greater than the CFST column one (7.29 for STR3 and 3.307 for CFST). However, under the eccentric load, strength of STR3 column was less than

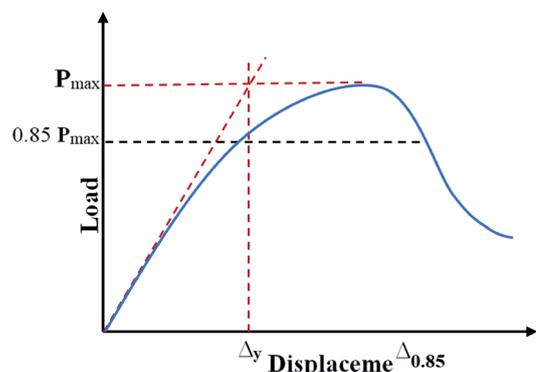


Fig. 8 Determination procedures of displacement ductility index

Table 8 Key test results in ABAQUS

Column type	Pu(kN) e = 0mm	Pu(kN) e = 50mm	Pu(kN) e = 75mm	Pu(kN) e = 100mm	$\mu_{0.85}$ e = 0mm	$\mu_{0.85}$ e = 50mm	$\mu_{0.85}$ e = 70mm	$\mu_{0.85}$ e = 100mm
STR3	3422	2194	1635	1258	7.29	7.35	8.65	6.78
MSTR	4335	2980	2342	1845	7.88	7.48	9.02	7
ASTR	4855	3614	2872	2400	8.125	10	10.666	8.18
CFST	3475	2256	1858	1559	3.307	6.25	6.667	6.28

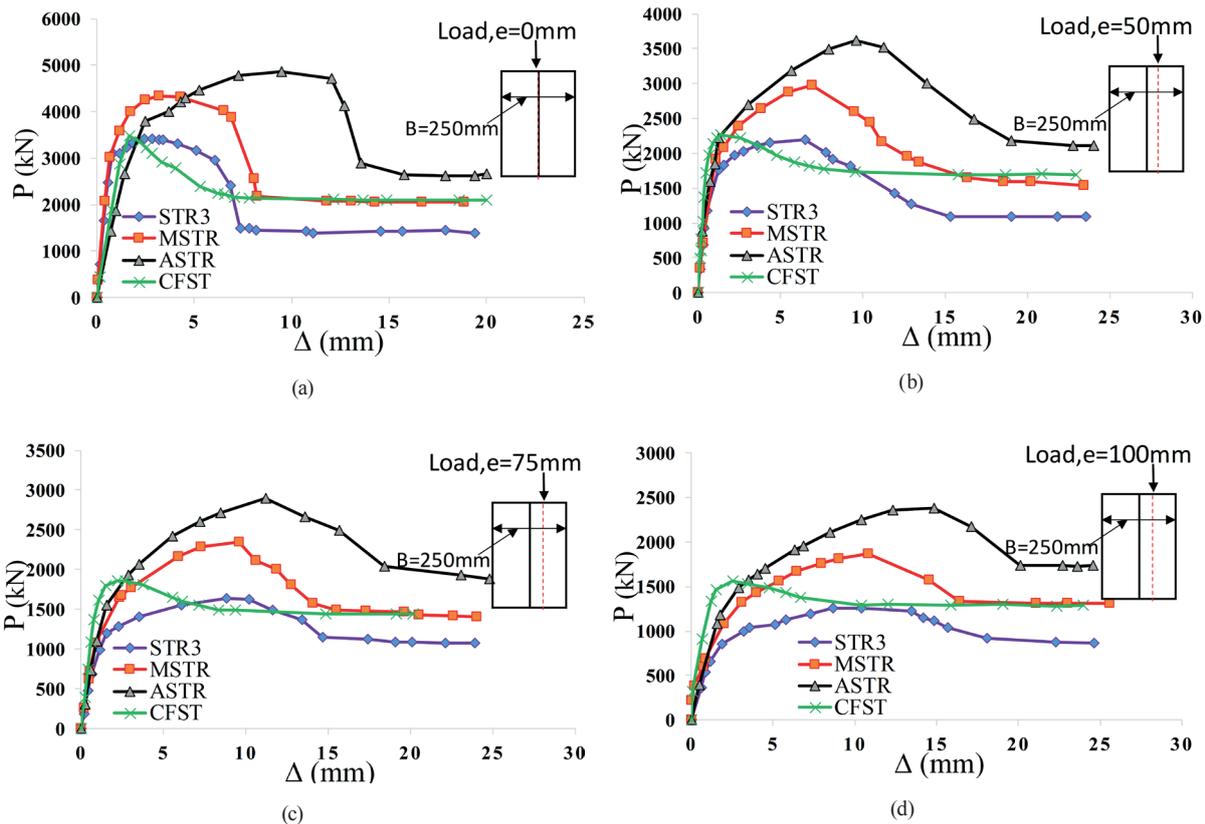


Fig. 9 Comparison of the P-Δ curve for columns under eccentric load at different distances

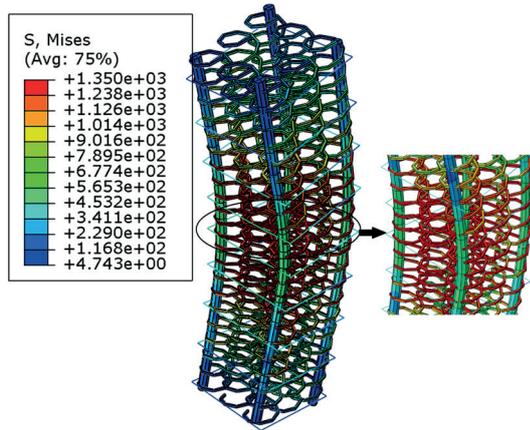
the CFST column. For instance, under the eccentricity of $e = 100$ mm, the bearing capacity of the STR3 column was 19 % smaller than that of the CFST column. As an interesting point, weight of the steel used in the CFST column is 3.48 times greater than STR3 column one.

According to the results presented in Fig. 9 and Table 8, the load capacity of the two columns MSTR and ASTR under pure compression load are 24 and 39 percent higher than that of the CFST column, respectively. Furthermore, under the eccentric load with eccentricity of 100 mm, the load-bearing capacity of MSTR and ASTR columns were 18 and 53 percent higher than that of CFST column, respectively. At the same time, the ductility of these columns (MSTR and ASTR) under the load with an eccentricity of 100 mm is 11 and 30 percent higher than the ductility of the CFST column, respectively. As an interesting point, the

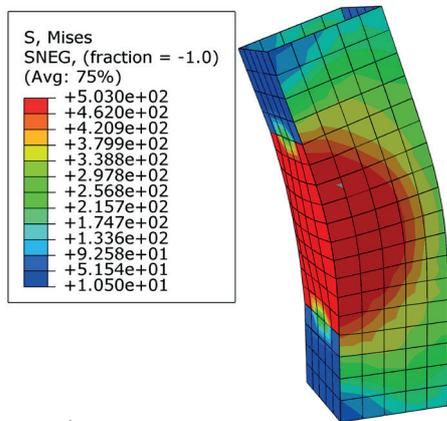
weight of the steel consumed in ASTR column is 12 percent lower than the CFST column. Fig. 10(a) and Fig. 10(b) show the Von-Mises stress distributions in the spiral reinforcement of MTSRT column and steel tube of CFST column, respectively (for load with eccentricity of 100 mm).

The energy absorption value (E) of the columns was calculated according to Fig. 11. According to Fig. 11, energy absorption value (E) defines as the area of the load-displacement curve up to the ultimate load [64, 65]. Table 9 shows the energy absorption of each column (E) under the pure axial and eccentric loads.

Table 9 indicates that the energy absorption of the STR column, which is the strongest among the MTSTR columns, is 10.96 times higher than that of the CFST column under the axial loading. Also, the energy absorption of the STR column under the eccentric load with an eccentricity



(a) von-Mises stresses-MTSTR



(b) von-Mises stresses-CFST

Fig. 10 Steel stresses developed within the CFST and STR columns

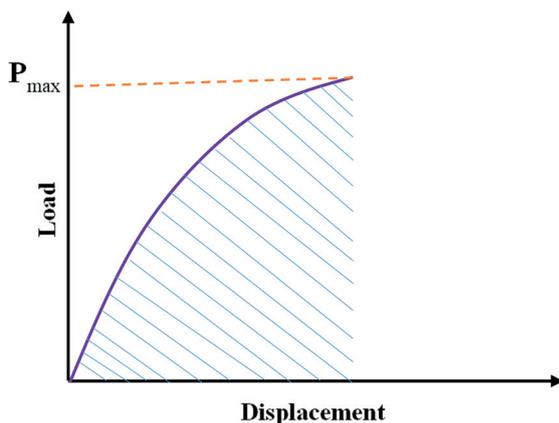


Fig. 11 Determination of energy absorption capacity

Table 9 Comparison of energy absorption of the columns

Column type	E (kN × mm) e = 0mm	E (kN × mm) e = 50mm	E (kN × mm) e = 75mm	E (kN × mm) e = 100mm
STR3	6659	12281	11917	10615
MSTR	12360	16445	17777	15421
ASTR	36801	30540	26346	27161
CFST	3355	2246	3034	2788

of 100 mm is 9.74 times greater than that of the CFST column with the same eccentricity. It should be noted that the weight of the steel used in the CFST column is 3.48 times greater than that of the STR3 column

5 Conclusions

This research focuses on the comparison between performances of two types of newly developed concrete columns under eccentric axial load using finite element method. This comparison was made between the bearing capacity, ductility and energy absorption of the MTSTR and CFST short columns. The results of this study showed that the load capacity, energy absorption and ductility of the MTSTR columns under the pure compression and eccentric compressive loads are more than those of the CFST column. The most important findings of this study are summarized as follows:

- The STR3 column is a type of the MTSTR columns contains eight circular spirals with a size of 4 mm, a diameter of 70 mm, a vertical spacing of 40 mm and finally, four corners placed longitudinal reinforcements with a diameter of 6 mm. The STR3 column shows a ductility of approximately two times higher than that of the CFST column under the axial compression. It is worth mentioning that the weight of the steel used in the CFST column is 3.48 times greater than that of the STR3 column (Fig. 9 and Table 8).
- Under the eccentric load, the STR3 column's bearing capacity is less than the CFST column. For instance, under the eccentricity of $e = 100$ mm, the bearing capacity of the STR3 column is 19 % smaller than that of the CFST column (Fig. 9 and Table 8).
- The MSTR column is a type of the MTSTR columns contains 8 circular spirals with a size of 6 mm, a diameter of 70 mm, a vertical spacing of 40 mm and finally, 4 corners placed longitudinal reinforcements with a diameter of 6 mm. The ASTR column is a type of the MTSTR columns contains 8 circular spirals with a size of 6 mm, a diameter of 70 mm, a vertical spacing of 40 mm and finally, 4 corners placed longitudinal reinforcements with a diameter of 20 mm. The load capacity of the MSTR and ASTR columns under pure compression is 24 and 39 percent higher than that of the CFST column, respectively (Fig. 9 and Table 8).
- ASTR column was detected as the most robust column among the MTSTR columns (STR, MSTR, and ASTR) in the current study.

- Under an eccentric load at a distance of 100 mm from the column center, the load-carrying capacity of the MSTR is 18 percent higher than that of the CFST column (Fig. 9 and Table 8).
- The bearing capacity of the ASTR column under the eccentric load with an eccentricity of 100 mm is 53 % more than that of the CFST column, while the weight of the steel consumed in ASTR column is 12 percent lower than the CFST column (Fig. 9 and Table 8).
- The ductility of the ASTR column under the eccentric load with an eccentricity of 100 mm is 30

percent higher than that of the CFST column (Fig. 9 and Table 8).

- The energy absorption capacity of the ASTR column under the eccentric load with an eccentricity of 100 mm is 9.74 times greater than that of the CFST column (Table 9).

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