

Displacement of the Buildings According to Site-Specific Earthquake Spectra

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Abstract

The probabilistic seismic hazard curves were based on appropriate attenuation relationships at rock sites with a probability of exceedance of 10% in 50 years in this study. Results from the model were compared to the response spectra proposed in Section 7 of TEC '07 and were found to differ in both amplitude and frequency content. The impact of these differences has been investigated with respect to building performance evaluation. Specifically, modal capacity diagrams and response spectra have been obtained for five buildings. Based on the diagrams and spectra, peak displacements have been calculated as well, revealing significant differences in the demand displacement curves of the buildings. As a result, damage estimates and predicted building performance will deviate from site specific performance to a greater degree. Using site-specific spectra and field data will be important for future earthquake-resistant design. One of the conclusions of the study is that the Code spectra do not offer a sufficient or comprehensive enough set of seismic demands and would lead to an under estimation of seismic hazard in the region of study. Therefore, site-specific design spectra for the region should be developed which reflect the characteristics of local sites.

Keywords

Seismic Hazard · Response Spectra · Lake Van · Peak Displacement

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1 Introduction

The seismic risk of building stock is of growing interest for academia as well as for governments due to the increasing urbanization and concentration of populations in earthquake prone and vulnerable areas. Since 1999 İzmit earthquake, Turkey has become recognized as one of the most earthquake-prone regions in the world. This is true considering that most of the country is mapped as having probabilities of peak ground acceleration PGA (up to 9.8 m/sec²).

Seismic hazard analysis of the earthquake-prone Eastern Anatolia region of Turkey has become more important due to its growing strategic importance as a global energy corridor and closer integration with the European Union. In this study Bitlis province is selected as the study area. The town of Bitlis, capital of the province, has a population of 70,000 (including surroundings) as of the year 2000. The town located 15 km away from Lake Van, along the steep slopes of the Bitlis River valley at an elevation of 1,400 m.

The seismicity of Bitlis has been evaluated using a performance-based earthquake engineering (PBEE) approach in this study. PBEE seeks to improve seismic risk decision-making through assessment and design methods that have a strong scientific basis and present options in terms that stakeholders can understand and make informed decisions. Given the inherent uncertainty and variability in seismic response, it follows that a performance-based methodology should be formalized within a probabilistic basis. The framework has four main analysis steps: Hazard analysis, structural/nonstructural analysis, damage analysis and loss analysis. The first assessment step entails a hazard analysis, through which one evaluates one or more ground motion Intensity Measures (IM). Standard earthquake intensity measures (such as peak ground acceleration or spectral acceleration) are obtained through conventional probabilistic seismic hazard analyses. Typically, IM is described as a mean annual probability of exceedance, which is specific to the location and design characteristics of the facility [1].

2 Local Geology and Seismicity of Bitlis

The local geological soil conditions change the characteristics of surface seismic response. It is a known fact that this may cause damage on the existing structures built on these grounds [2]. The Lake Van Basin which contains Bitlis is located in the region known as the Bitlis Thrust Zone in geological terms. It is a collapsed tectonic basin which is related to the Eastern Taurus region [3]. Orogenic movements have occurred in the field until third phase of Miocene. Volcanic events have caused many faults to form, as well as depressions and large lakes in this period [4, 5]. Metamorphic rock in the region belonging to the Bitlis Massif include the Upper Cretaceous Ahlat-Adilcevaz mélangé and Ahlat conglomerate, Miocene Adilcevaz limestone, Pliocene-Quaternary volcanic rocks and alluvial deposits form the surface in Bitlis and surrounding region [6, 7]. A geological map of Lake Van Basin is shown in Fig. 1.

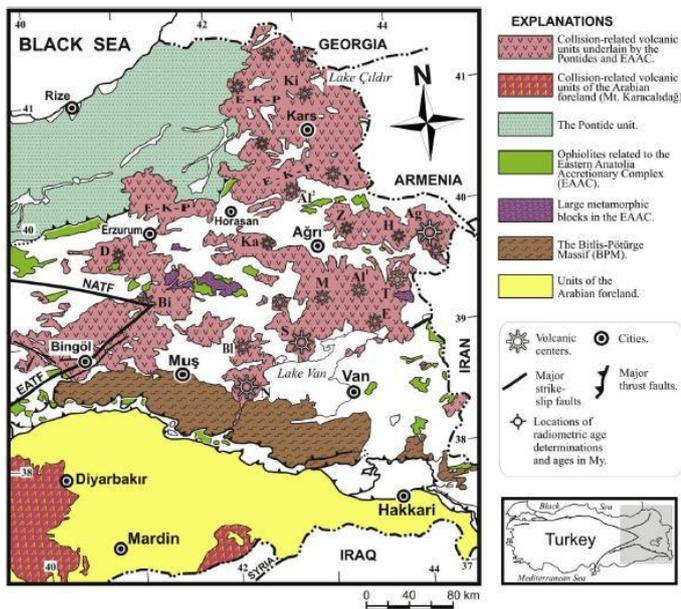


Fig. 1. Geological map of the Lake Van region. N – Nemrut Volcano, S – Süphan Volcano in the immediate vicinity of the lake. EATF – East Anatolian Fault; NATF – North Anatolian Fault [8]

The general tectonic setting of Eastern Anatolia is controlled mainly by the collision of the Northerly-moving Arabian plate with the Anatolian plate along a deformation zone known as the Bitlis Thrust Zone (Fig. 2, Arrow below Bitlis Zagros Suture Zone). The collision drives the westward extrusion of the Anatolian plate along two well known transform faults with known as the Bitlis Thrust Zone (Fig. 2, Arrow below Bitlis Zagros Suture Zone). The collision drives the westward extrusion of the Anatolian plate along two well know transform faults with different slip directions, the right-lateral North Anatolian (NAFZ) and the left-lateral East Anatolian Fault (EAFZ) zones, which join each other in Karlıova Triple Junction (KTJ) in eastern Anatolia (Fig. 2, letter K). To the east of KTJ, however, the compressional deformation is largely accommodated within the Eastern Anatolian Block through distributed NW-SE trending right-lateral faults and NE-SW trending left lateral faults representing

escape tectonics, and shortening of the continental lithosphere along the Caucasus thrust zone. East-west trending Mush-Lake Van and Pasinler ramp basins constitute other conspicuous tectonic properties within the eastern Anatolia [9–13].

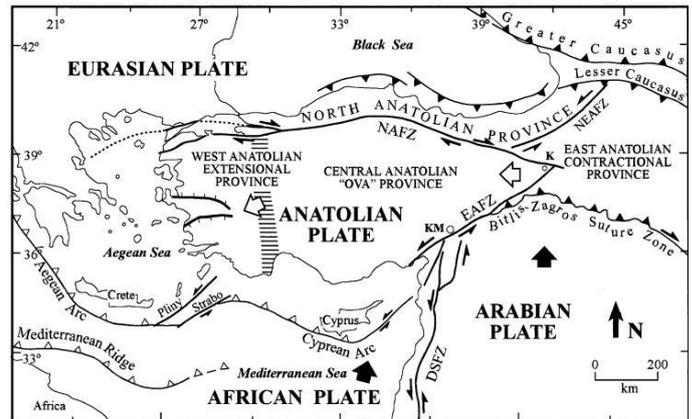


Fig. 2. Tectonic map of Turkey including major structural features [14]

The Lake Van basin has been seismically active region as indicated by historical sources. Table 1 tabulates the significant earthquakes occurred in Bitlis and surrounding area before 20th century.

Based on historical and instrumented earthquakes, Bitlis is constantly under the influence of both micro- and macro- earthquakes. Thus, it will not be difficult to say that Bitlis will remain under the influence of larger earthquakes [15]. Bitlis Centre City is in first degree of seismic zones in the current seismic hazard map of Turkey with a minimum effective peak horizontal ground acceleration of 0.40 g.

3 Site-Specific Design Spectra for Bitlis Province

The seismic hazard analysis approach is based on the model developed originally by Cornell (1968) [16] who quantified hazard in terms of the probability of exceedance of a peak ground acceleration (PGA). The procedure for conducting a probabilistic seismic hazard analysis includes characterizing the seismic source, determining size distribution and rate of occurrence estimating ground motion, and lastly, analyzing probability.

In the current study, since the neotectonic faults are not identified in the research area clearly, earthquake sources are characterized as area source zones. Area seismic sources are often defined where specific fault data are not known, but seismicity does exist. Area sources assume that the rate of occurrence is uniform throughout. Therefore, every location within the area has equal probability that an event will occur. All seismic sources, that can generate strong ground shaking in Bitlis and surroundings, are classified into 7 areal seismic zones (Fig. 3.): (1) Bitlis Zagros Suture zone; (2) Northern Bitlis thrust fault zone [17]; (3) Kavakbaşı Fault zone; (4) Malazgirt fault zone; (5) Ahlat and surrounding fault zone; (6) Suphan Fault zone ; and (7) Southern Van faults (Erçek fault, Kalecik fault, Edremit fault and Southern Boundary fault [18].

Tab. 1. The significant earthquakes in and around Bitlis before 20th century

No	Date	Lat. (°)	Lan. (°)	Location	M	I
1	461	39.10	42.50	Malazgirt		X
2	1012	39.10	42.50	Malazgirt		VII
3	1101	38.50	43.50	Ahlat - Van		VI
4	1110	38.50	43.50	Ahlat - Van		VIII
5	1111	38.50	42.70	Ahlat - Van		IX
6	1208	38.70	42.50	Ahlat-Van-Bitlis-Muş	6,5	
7	1245	38.74	42.50	Ahlat - Bitlis- Van - Muş		VIII
8	1246	38.90	42.90	Lake Van (Ahlat - Erçiş -Van)		VIII
9	1275	38.40	42.10	Bitlis- Ahlat -Erciş – Van		VII
10	1276	38.90	42.50	Bitlis- Ahlat -Erciş – Van		VIII
11	1282	38.90	42.90	Ahlat – Erçiş		VII
12	1345	39.10	42.50	Malazgirt		VIII
13	1363	38.70	41.50	Muş andsurrounding		IX
14	1415	38.50	43.00	Van Gölü		V
15	1439	38.50	42.10	Nemrut		VI
16	1441	38.35	42.10	Nemrut		VIII
17	1444	38.50	43.40	Nemrut - Van		VI
18	1546	38.50	43.40	Van - Bitlis		V
19	1582	38.35	42.10	Bitlis and surrounding		VIII
20	1646	38.50	43.40	Van and surrounding		VII
21	1647	39.15	44.00	Van - Muş -Bitlis		IX
22	1648	38.30	43.70	Van and surrounding	6,7	VIII
23	1670	38.00	42.00	Hizan - Siirt	6,6	
24	1682	38.40	42.10	Bitlis		
25	1696	39.10	43.70	Çaldıran - Bitlis	6,8	X
26	1701	38.50	43.40	Van and surrounding		VIII
27	1704	38.50	43.40	Van		VII
28	1705	38.40	42.10	Bitlis	6,7	IX-X
29	1715	38.70	43.50	Van - Erçiş	6,6	VIII
30	1869	38.40	42.10	Bitlis andsurrounding		VII
31	1871	38.50	43.40	Van -Nemrut	5,5	VII
32	1881	38.50	43.40	Van andsurrounding	7,3	IX
33	1884	37.50	42.50	Bitlis - Pervari	6,9	
34	1891	38.80	42.50	Malazgirt- Adilcevaz-Bitlis	5,5	VIII
35	1892	39.10	42.50	Malazgirt - Muş		VII

On any given fault within any given region, earthquakes occur at irregular intervals in time, and one of the basic activities in seismology has long been the search for meaningful patterns in the time sequences of earthquake occurrence [19]. Among a number of recurrence laws have been proposed, in this study, Gutenberg and Richter [20] law was used due to the fact that there is no available evidence to determine whether the Gutenberg –Richter or some other recurrence laws are correct. During any given interval in time, the general underlying pattern or distribution of size of events is that first described by Gutenberg and Richter, who derived an empirical relationship between magnitude and frequency of the form;

$$\log N = a - bM \quad (1)$$

where N is the number of shocks of magnitude at least M per unit time and unit area, and a and b are seismic constants for any

given region [19].

In a seismic hazard modelling study of Bitlis, recurrence rates are estimated by using historical and digital records given a partial list in Table 1 and instrumental data. After the compilation of collected data, a plot of “ M ” against “ $\log N$ ” was constructed and the best-fit line of the form of Eq. (1) was determined by regression analysis (Fig. 4).

In probabilistic seismic hazard analysis, beside magnitude–frequency relationship which is calculated for Bitlis province as $\log N = 5.6247 - 0.7794 M$, a relationships between magnitude and fault rupture parameters of length L_{sub} (km), width W (km), area A (km²) and displacement D (m) is also required. In a study of a worldwide database of 244 earthquakes, for strike-slip fault types Wells and Coppersmith (1994) [21] obtained:

$$M_w = 4.33 + 1.49 \log L_{sub} S = 0.2 \quad (2)$$

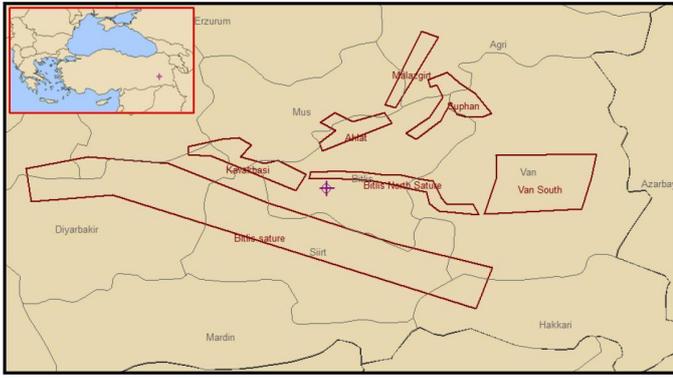


Fig. 3. Earthquake are all zones (Bitlis Suture, Van South, Bitlis North Sature, Kavakbasi, Ahlat, Malazgirt and Suphan) in Bitlis and surroundings

where s is the residual standard deviation.

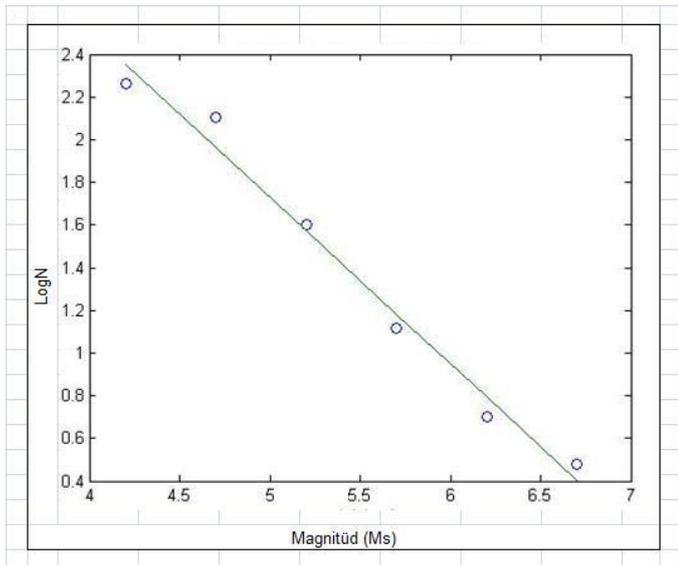


Fig. 4. Gutenberg-Richter magnitude–frequency relationship for earthquakes from Bitlis and surrounding data

In Eastern Anatolia region, previously recorded strong ground motion acceleration records are limited. Therefore, in the current analysis, worldwide applicable three empirical attenuation relationships are utilized to perform the seismic hazard analysis. Attenuation relationships for rock sites employed in this study are Abrahamson-Silva (1997) [22], Ambraseys et al. (2005) [23], Boore-Joyner-Fumal (1997) [24], Campell (2003) [25] and Idriss(2008) [26] (Fig. 5).

After the compilation of the seismic hazard analysis data, the procedure for conducting a probabilistic seismic hazard analysis, by using EZ-FRISK [27, 28] software, was employed to produce the PGA as a function of return periods (Fig. 6), and uniform probability response spectra for selected return periods (Fig. 7) The results of probabilistic seismic hazard analysis for Bitlis are presented in terms of spectral responses at 5% damping for the return periods of 72, 474.6 and 2474.9 years (Fig. 7). The results are compared with the spectral responses proposed for seismic evaluation and retrofit of building structure in Turkey Earthquake Code (2007) Section 7 [29]. The results of probabilistic seismic hazard analyses revealed peak acceleration val-

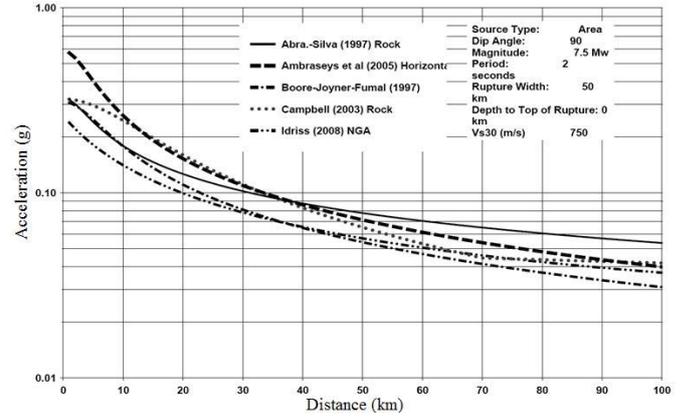


Fig. 5. Abra-Silva (1997), Ambraseys et al (2005), Boore-Joyner-Fumal (1997), Campell (2003) and Idriss (2008) attenuation relationships for rock sites.

ues for a typical rock site as 0.76 g for 50% probability of exceedance in 50 years, 1.61 g for 10% probability of exceedance in 50 years and 2.68 g for 2% probability of exceedance in 50 years. The obtained results are compared with the spectral responses proposed for seismic evaluation and retrofit of building structure in Turkey Earthquake Code, Section7 (Fig. 7).

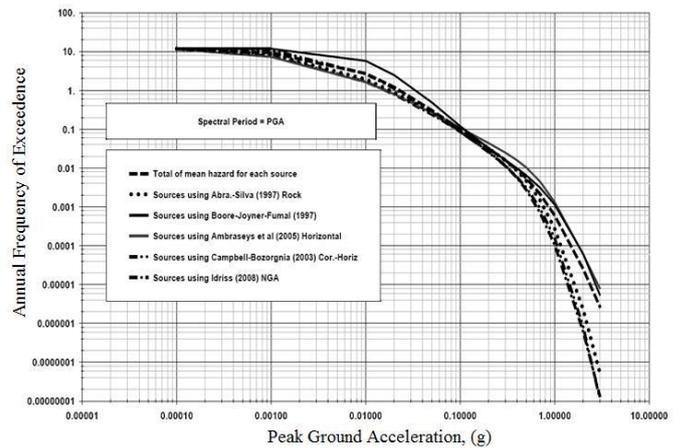


Fig. 6. Peak ground acceleration (PGA) at Bitlis with varying return periods.

4 Calculation of Displacement of the Buildings According to Spectra

During the last two decades, performance based design and assessment methods have become rather more popular than during the era they were firstly proposed. In the near future, it is likely that when new generation seismic codes are released, performance based approach will be the most common tool for the design of new structures. Currently, however, performance based design tools suffer from a major drawback that their presentation of the seismic behavior is restricted by a single mode response. Therefore such methods can be reliably applied only to the two-dimensional response of low-rise, regular buildings.

The demand spectra that were used for determining the performance of buildings systems have shown the maximum response to earthquake ground motion during an earthquake. In performance based design and assessment methods the earth-

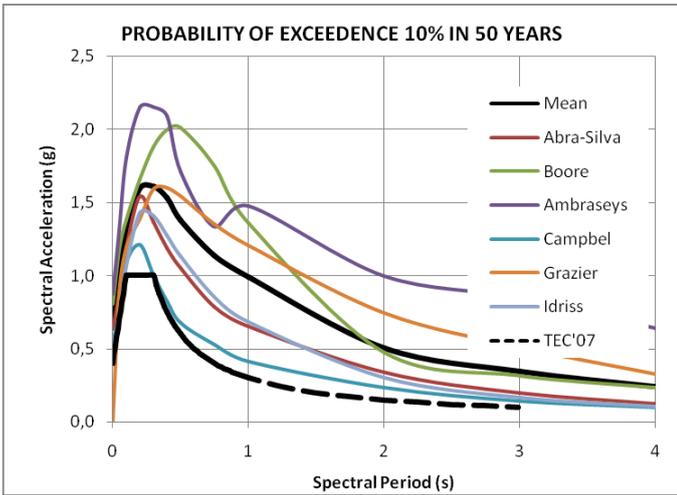


Fig. 7. Comparison of Spectral responses at 5% damping for the return period of 474.6 years in Bitlis

quake demand is first calculated. It is then necessary to determine the structural performance by comparing these demand values to deformation capacity for the selected performance levels. Building evaluations were performed separately for the spectrum obtained from the seismic hazard analysis and for design spectra that has been given in TEC'07. Modal capacity diagrams and response spectra have been obtained for five buildings. Peak displacements for these five buildings have been calculated based on capacity diagrams and response spectra. The displacement demands were calculated by using the equivalent displacement rule given in the TEC'07. Modal capacity diagram with coordinates given as “a (acceleration) – d (displacement)” and response spectra with coordinates given as “ S_a (spectral acceleration) – S_d (spectral displacement)” are shown graphically for each building studied in Fig. 8 - 17.

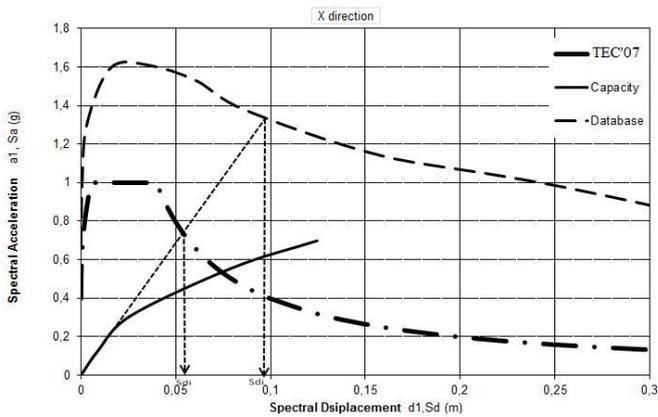


Fig. 8. Modal Capacity – Response Spectra diagrams for Building 1 in the X direction

The comparison of building peak displacements was given in Table 2.

5 Conclusions

By utilizing available data improved methods, a probabilistic seismic hazard analysis of Bitlis province in Turkey was performed. As a first step toward performance based earthquake

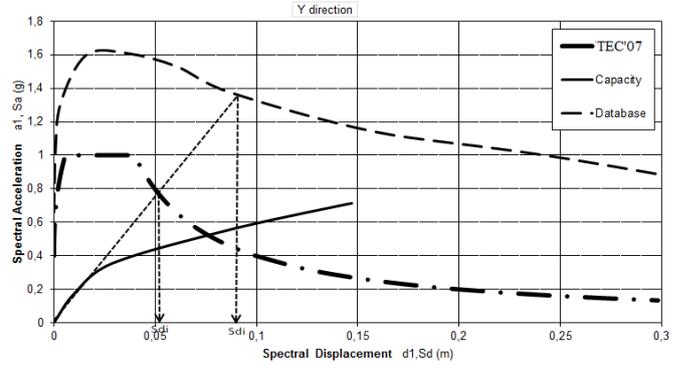


Fig. 9. Modal Capacity – Response Spectra diagrams for Building 1 in the Y direction

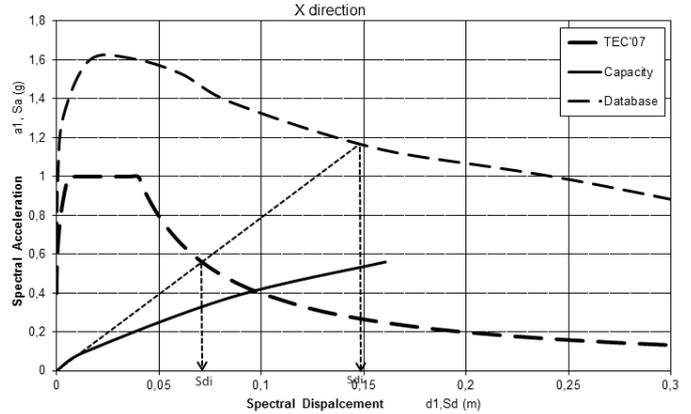


Fig. 10. Modal Capacity – Response Spectra diagrams for Building 2 in the X direction

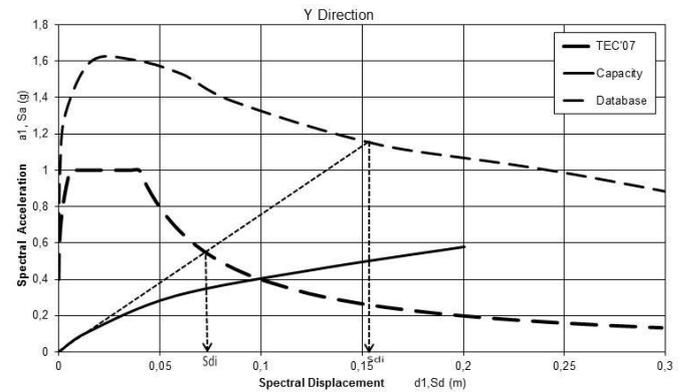


Fig. 11. Modal Capacity – Response Spectra diagrams for Building 2 in the Y direction

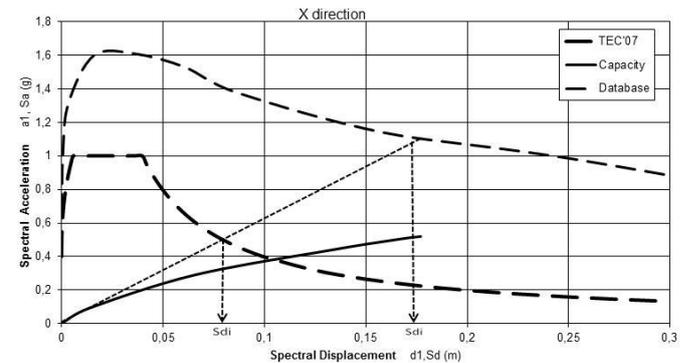


Fig. 12. Modal Capacity – Response Spectra diagrams for Building 3 in the X direction

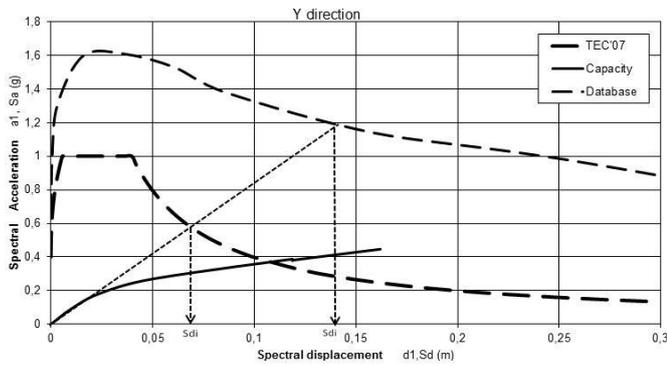


Fig. 13. Modal Capacity – Response Spectra diagrams for Building 3 in the Y direction

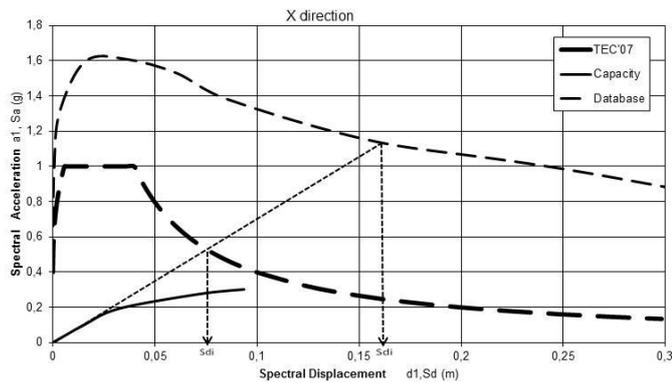


Fig. 14. Modal Capacity – Response Spectra diagrams for Building 4 in the X direction

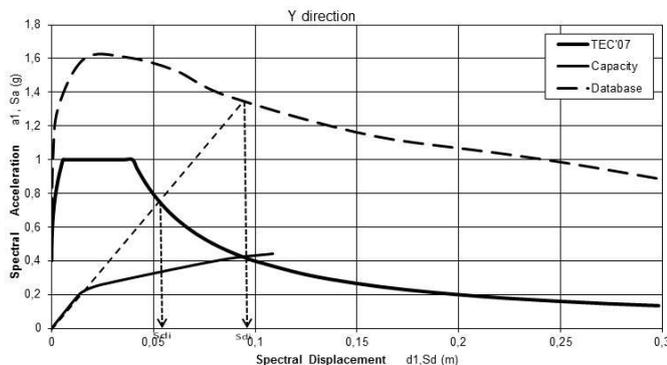


Fig. 15. Modal Capacity – Response Spectra diagrams for Building 4 in the Y direction

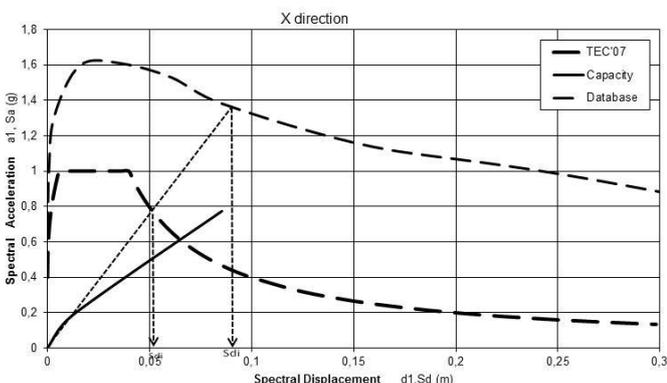


Fig. 16. Modal Capacity – Response Spectra diagrams for Building 5 in the X direction

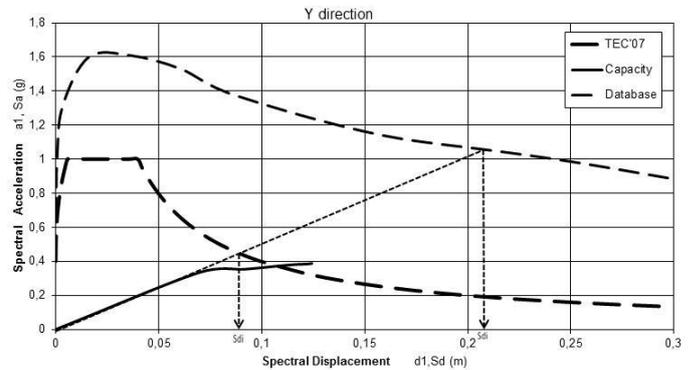


Fig. 17. Modal Capacity – Response Spectra diagrams for Building 5 in the Y direction

Tab. 2. Building's peak displacements for TEC'07 and the response spectra obtained from this study

Building Number	Direction	TEC'07		DATABASE	
		S_{del}	u_{N1}	S_{del}	u_{N1}
1	X	0,056	0,073	0,09	0,1171
	Y	0,052	0,067	0,085	0,1101
2	X	0,072	0,0923	0,148	0,1897
	Y	0,079	0,1046	0,152	0,2013
3	X	0,08	0,1073	0,17	0,2281
	Y	0,069	0,091	0,142	0,1874
4	X	0,079	0,0998	0,157	0,1984
	Y	0,051	0,066	0,092	0,1195
5	X	0,053	0,0675	0,091	0,1159
	Y	0,088	0,113	0,206	0,2655

engineering, it is well understood that the Code-proposed spectra are not sufficient to represent earthquake demand in the performance evaluation. The results of this work will form the basis for the replacement of the existing earthquake design spectra in evaluation of earthquake performances of the existing buildings in Bitlis province.

In this study, since active faults are not identified clearly, regional areas were used as an earthquake source zones. Future work will increase the resolution of the seismotectonic model by adding specific active faults. The obtained results are compared with the spectral responses proposed for seismic evaluation and retrofit of building structure in Turkish Earthquake Code, Section 7 and the amplitude and frequency range was different from each other. Modal capacity diagrams and response spectra have been obtained for five buildings. Peak displacements for these five buildings have been calculated based on capacity diagrams and response spectra. Results show that there were significant changes in the demand displacement of buildings. Therefore, damage estimates and building performance will better reflect real values for the buildings which did not meet the demand displacement (Fig. 18).

Using specific spectra obtained from site-specific investigation will be important for earthquake-resistant design of structures. At the end of this study, it is anticipated that for the performance evaluation of the existing structures, the Code-proposed earthquake response spectra are not sufficient and current esti-

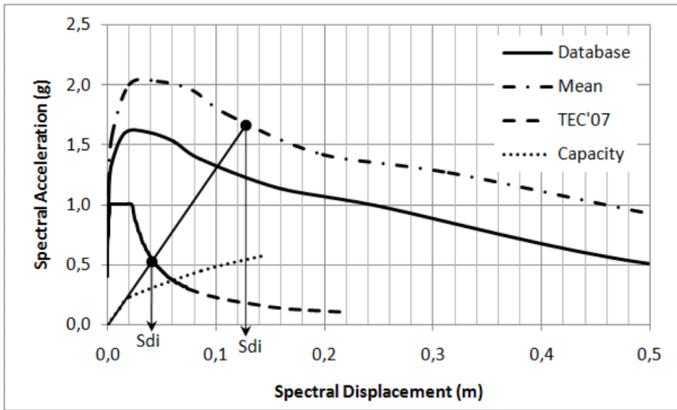


Fig. 18. Comparison of demand displacement of the earthquake

mations show that the potential seismic hazard in this area of the Turkey is underestimated by the code. Therefore, site-specific design spectra for the region should be developed, especially for local sites.

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